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RIX'S CREEK PTY LIMITED

EFFECTS OF BLASTING IN THE RIX'S CREEK CONTINUATION PROJECT AREA

REVISION 2018-2

Effects of Blasting in Current (2018) Proposed Blasting Areas

Alan B Richards
28th February, 2018

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1 INTRODUCTION

Terrock Consulting Engineers were requested by Rix's Creek Pty Limited (RCPL) to investigate the effects of blasting in the continuation area located in the West Pit (SW of the New England Highway), and in a smaller area in the North Pit (NW of the New England Highway), and this was covered in previous reports prepared by Terrock.

In March 2017, Terrock provided an initial assessment for the continuation project which formed part of the initial revised response to submissions. During the period between receipt of the report and submission of the Revised Response to Submissions, Rix's Creek Mine purchased two of the privately owned receivers where blast monitoring was being undertaken, which had been assessed within the March Report. Subsequently blast monitoring locations were revised following acquisition and in February 2018, Terrock were requested by RCPL to prepare a revised report that provides assessment of the effects of blasting on the revised privately owned receivers.

This revised report provides an update on ground vibration and airblast overpressure levels that will result from the current (February 2018) proposed blasting areas, using the same methodology that was used in previous reports. These changes are included in Sections 7.1 and 7.2 of this updated report, and in the appendices. No change has been made to the sections of the report dealing with all other environmental issues.

The Location Plan in **Figure 1** which shows:

- Current extraction area;
- Both continuation areas;
- Sites sensitive to blast vibration (residential areas);
- Current monitoring locations;
- The New England Highway;
- Historic Coking Oven remains.

Currently the blast vibration is monitored in accordance with current consent conditions at:

- Wright Residence – Maison Dieu Rd
- Mines Rescue – Singleton Heights
- Retreat – Bridgeman Hill
- Watling Residence.

2 ENVIRONMENTAL BLASTING RELATED ISSUES

The related issues for blasting in the continuation areas are:

- Ground vibration control;
- Airblast control;
- Flyrock control;
- Dust and Fume Management;
- Traffic management on New England Highway;
- Stability of the New England Highway;
- Protection of the Historic Coking Oven remains.

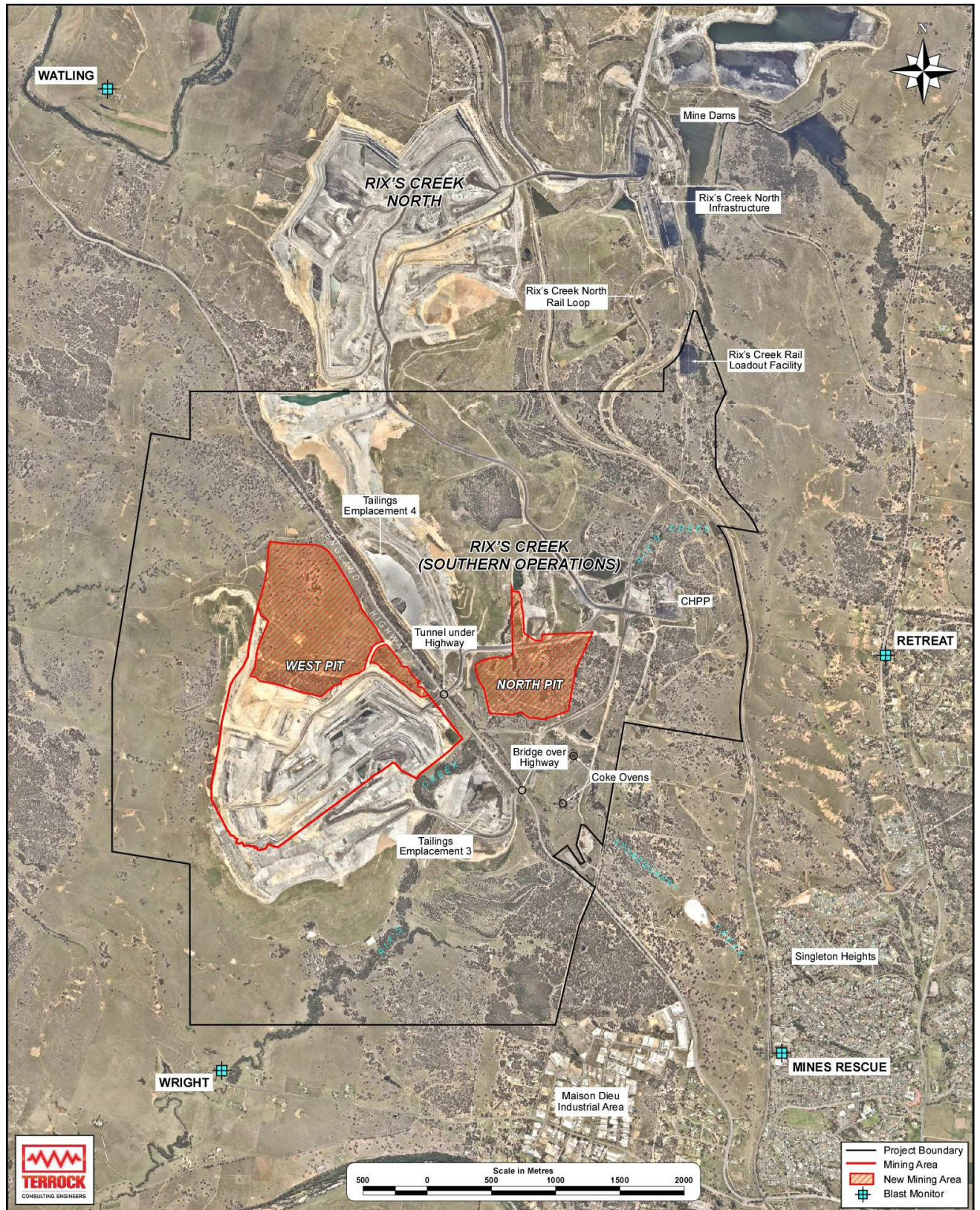


Figure 1 – Location Plan

3 REGULATORY BLAST VIBRATION LIMITS

3.1 The nature and measurement of blast vibration

When an explosive charge is fired, explosive energy produces the following effects:

- Rock shattering and displacement
- Ground transmitted blast vibration - (ground vibration)
- Air transmitted blast vibration - (airblast overpressure)

3.1.1. Ground Vibration

Ground vibration radiates outwards from the blast site and gradually reduces in magnitude, in the same manner as ripples behave when a stone is thrown into a pool of water, schematically shown in **Figure 3.1**. The motion of the wave can be defined by taking measurements of a float on the surface of the water. With suitable instruments we can measure the displacement or amplitude, the velocity, the acceleration of the float and the wave length of the waves.

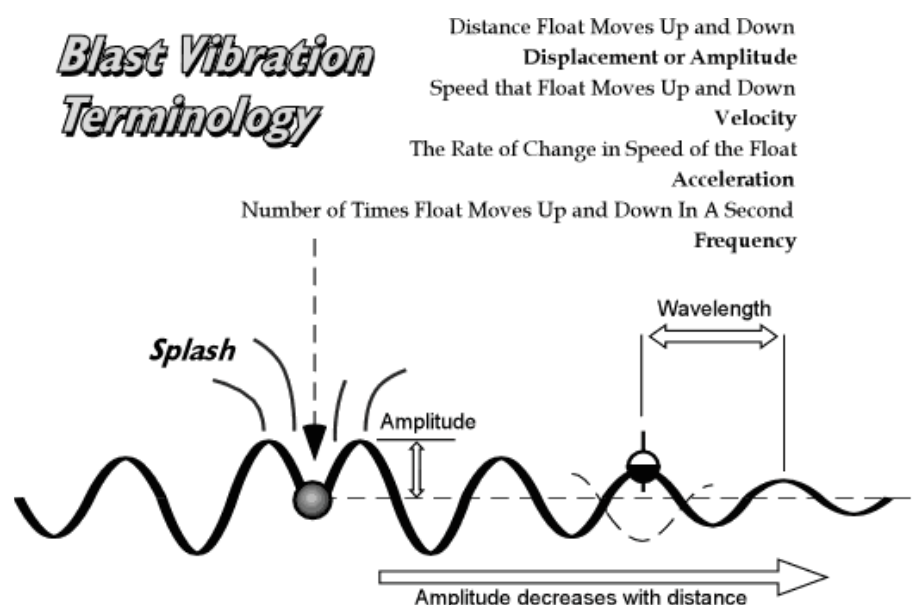


Figure A1.5 - Schematic diagram of vibration terminology

With ground vibration, the motion of the surface of the ground can be measured by coupling a suitable instrument directly to the surface.

For regulatory purposes, it has become common practice to measure ground vibration using a seismograph with a geophone securely attached to the ground.

The geophone measures the velocity that a point (or particle) on the ground moves in three dimensions at the measurement location as the vibration waves pass.

This is called the particle velocity, and the maximum value is called the peak particle velocity (PPV), measured in terms of millimetres per second (mm/s).

To define the motion in three dimensions, it is necessary to use three transducers to measure the vibration in three mutually perpendicular directions and then determine a Peak Particle Velocity or Peak Vector Sum (PVS), which is the instantaneous maximum vector of the three individual measurements:

$$\text{ie. PPV (PVS)} = \sqrt{v_t^2 + v_l^2 + v_v^2}$$

Ground vibration from blasting must be measured with a blast vibration meter that complies with the requirements of AS2187.2 – 2006.

3.1.2. Airblast Overpressure

When air transmitted vibration is within the range of hearing it is called sound (with frequencies in the range 20 Hz to 200,000 Hz). When its frequencies are below the range of hearing is generally referred to as concussion or airblast.

Noise is generally measured with a sound level meter that simulates the ear by filtering out frequencies below 20 Hz, the results obtained are specified as decibels (A), or dBA.

Airblast overpressure is substantially sub-audible. Although these frequencies are below the range of hearing they affect structures, and the response of the structures can be sensed by people who are inside. This explains why a blast that is barely noticed outside can be noticed by people inside a building.

Airblast overpressure is measured with special sound level meter that does not filter out the low frequencies below 20 Hz that affect structures, and the results obtained are specified as decibel (linear), or dBL.

Airblast overpressure must be measured with a meter that complies with the requirements of AS2187.2-2006.

3.2 Human and Structure Response

3.2.1. Human Response

Humans are more sensitive to blast vibration than structures, and this has resulted in human response limits that are well below levels that will cause damage to structures.

Human response to blast vibration, which is based by the experience of the Terrock personnel over a period of 40 years, is summarised in the table below:

| | Ground Vibration | Airblast Overpressure |
|---|------------------|-----------------------|
| Threshold of human response | 0.1 to 0.5 mm/s | 90 to 100 dBL |
| Levels that acceptable to most people and not result in complaint. | Up to 2 mm/s | Up to 110 dBL |
| Levels that are likely to cause complaint. | 2 to 5 mm/s | 110 to 115 dBL |
| Levels that will result in an increased number of complaints. | 5 to 10 mm/s | 115 to 120 dBL. |
| Levels that are generally unacceptable to the Australian community. | Above 10 mm/s | Above 120 dBL. |

3.2.2. Structure Response

Structural damage will occur at levels that are well above levels that are considered unacceptable to humans.

Authoritative research (ref ACARP Project C.9040 – Effect of Blasting on Structures) shows that at a ground vibration level of 10 mm/s, the stress induced into a brick veneer house is less than 10% of the strength of the weakest structural element (the interior plasterboard).

AS2187.2-2006 includes recommended ground vibration and airblast overpressure limits for damage control. These structural limits are well above the human response limits specified in environmental licences and development consents.

It should be noted that AS2187.2-2006 does not include a specific limit for historic structures. Appropriate limits for historic structures should be assessed on an individual case basis.

3.3 Development Consent Conditions

The following human ground vibration and airblast limits are specified in the current Development Consent conditions.

| | |
|-------------------------|---|
| Ground Vibration | ≤ 5 mm/s for 95% of blasts in a 12 month period ≤ 10 mm/s for all blasts |
| Airblast | ≤ 115 dBL for 95% of blasts in a 12 month period ≤ 120 dBL for all blasts. |

These human response limits are based on the Australian & New Zealand Environmental Council (ANZEC) “Technical Basis for Guidelines to Minimise Annoyance due to Blasting Overpressure and Ground Vibration”. This publication specifies the following guideline limits at sensitive sites:

- Ground vibration: 5 mm/s for 95% of blasts within a 12 month period, with exceedance permitted to 10 mm/s for 5% of blasts.
- Airblast overpressure: 115 dBL for 95% of blasts within a 12 month period, with exceedance permitted to 120 dBL for 5% of blasts.

4 DESCRIPTION OF THE BLASTING ENVIRONMENT

Blasting of overburden and interburden is necessary to break the rock to enable it to be removed and the coal seams beneath uncovered. The thickness of the rock layers varies considerably from about 2m to over 30m in the Northern Pit. In the Western Pit the thickest interburden blasting is currently 35m but this may increase to over 40m as the pit advances to the North West.

Geological cross sections through both pits are shown in **Figures 2a & 2b**. Individual blasts are designed and the specifications altered to comply with the regulatory environmental ground vibration and airblast limits at the nearby sensitive sites.

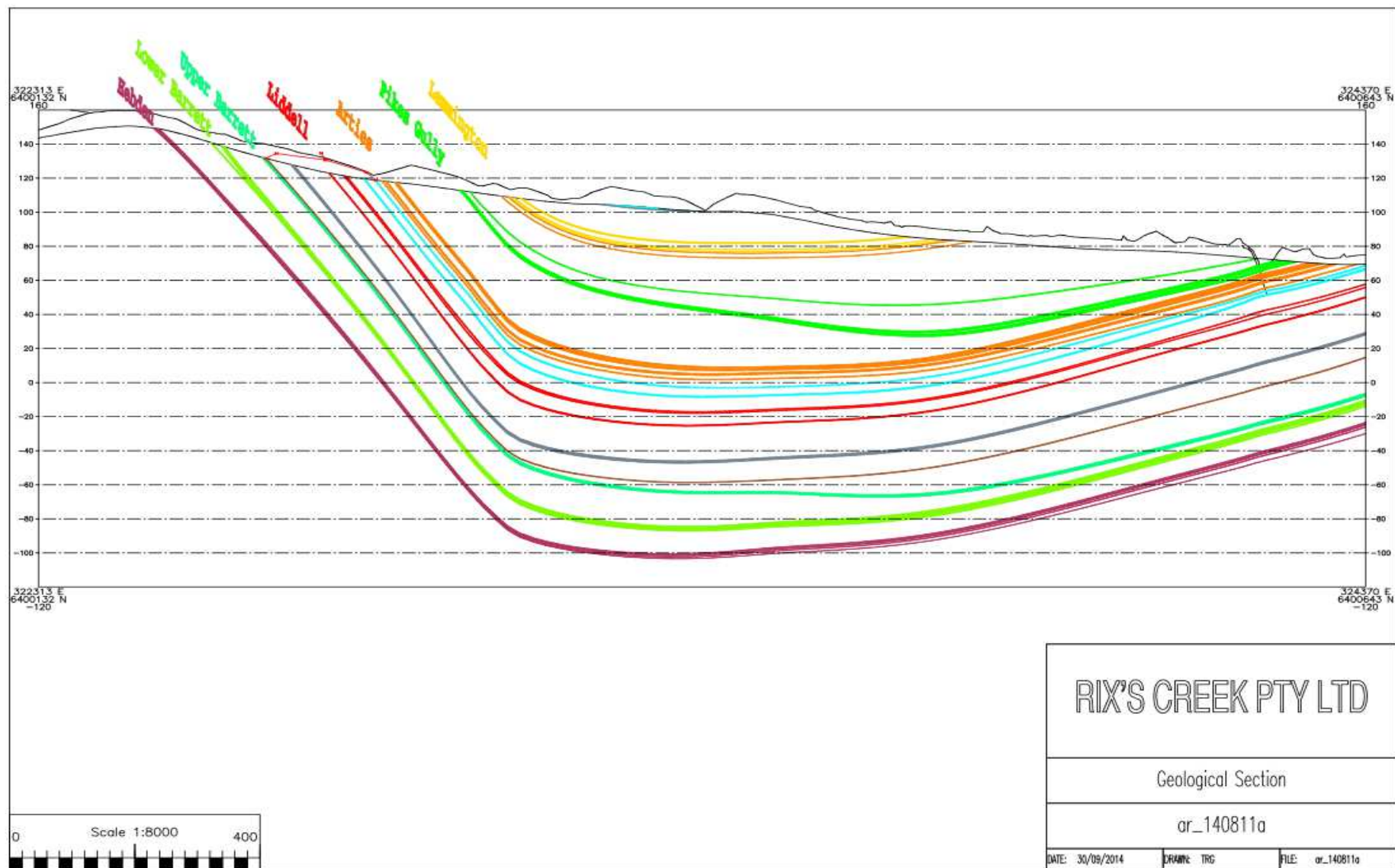


Figure 2a – Geological Cross Section – North Pit

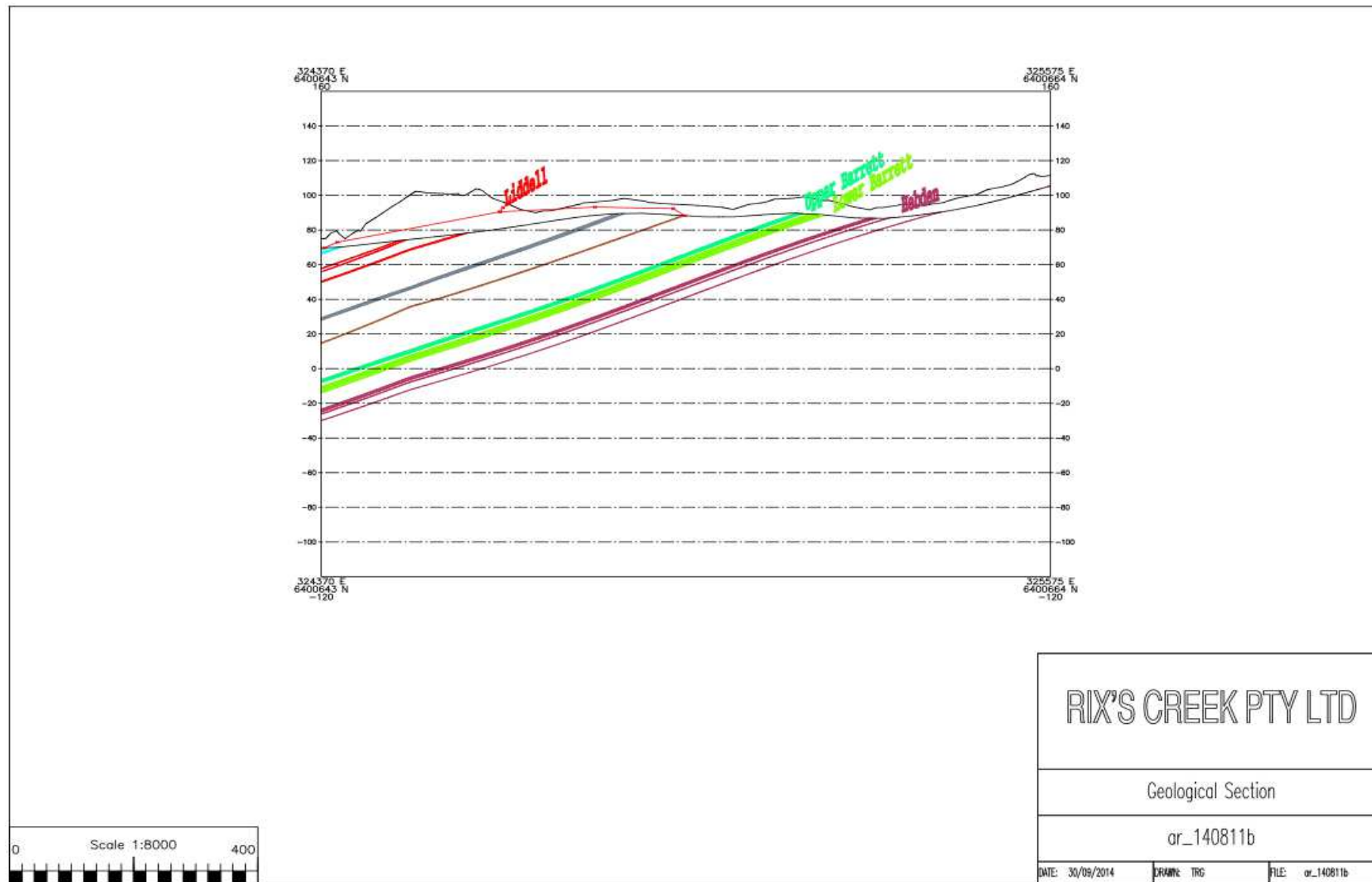


Figure 2b – Geological Cross Section – West Pit

5 DESCRIPTION OF BLASTING PRACTICE

Blasting practice uses environmental blast design principles to adjust the blasting specifications to control airblast and ground vibration to regulatory limits as well as controlling flyrock. To control airblast and flyrock in the shallow partings, the shallowest holes that can be fired are about 2.5m deep and a typical loading would be 0.2 – 0.3m explosives and 2.2 – 2.3m of stemming. In deeper blasts, the burden, spacing and stemming height are varied to achieve the ground vibration and airblast targets.

A range of explosives are also used with densities varying from 0.8 g/cc (ANFO) to 1.1 g/cc (Heavy ANFO 1.1) to 1.3 g/cc (HA 1.3). The explosive is chosen after consideration of the rock blastability and the presence of water in the blast holes.

A typical range of blasting specifications used in the Open Cuts is shown in **Table 1**.

Table 1 – Open Cut nominal blasting specifications

| | | | | | | | |
|---------------------------|--------------------|---------------------|-----|-----|-----|-----|------|
| Blast hole diameter: | | 229mm | | | | | |
| Face Height (hole depth): | | 2.5 m – 35m | | | | | |
| Stemming height: | | 2.2 m – 5m | | | | | |
| Explosive column: | | 0.3 – 30 m | | | | | |
| Explosive | Charge mass/m (kg) | Hole Depth (m) | 2.5 | 10 | 15 | 20 | 35 |
| | | Stemming Height (m) | 2.2 | 4.0 | 4.5 | 4.5 | 5.0 |
| ANFO | 32.6 | Charge Mass (kg) | 10 | 195 | 342 | 505 | 978 |
| HA 1.1 | 44.0 | Charge Mass (kg) | 13 | 264 | 462 | 682 | 1320 |
| HA 1.3 | 53.0 | Charge Mass (kg) | 16 | 318 | 556 | 821 | 1500 |

The specifications may be varied following environmental blast design to ensure compliance with the regulatory limits.

6 BLAST VIBRATION FROM CURRENT BLASTING OPERATIONS

The range of PPV and Peak Airblast for 2017 at the regulatory monitoring stations is summarised in **Table 2**.

Table 2 – Summary of PPV and Peak Airblast - 2017

| | | Watling | Retreat | Mines Rescue | Wright |
|------|------------|-------------|-------------|--------------|-------------|
| 2017 | | | | | |
| | PPV (mm/s) | 0.01 - 0.83 | 0.01 - 0.84 | 0.02 - 1.02 | 0.08 - 1.92 |
| | AOP (dBL) | 85 - 110 | 83- 115 | 80 - 110 | 83 - 110 |

The ground vibration resulting from all blasts in the investigation period was well below the regulatory limit of 5 mm/s (95%) at all monitoring stations. The airblast overpressure resulting from all blasts did not exceed regulatory limits at all monitoring stations.

7 BLAST ANALYSIS

The blast vibration monitoring results for 2017 have been analysed to determine what is currently being achieved and how this transfers into the continuation area.

7.1 GROUND VIBRATION

There is a considerable variation in the blasting depths (2m to 35m) and the resulting charge mass. The centroidal contour approach was considered to be the best method to demonstrate the worst case ground vibration situation.

7.1.1. Centroidal Contour Approach

This approach is used to demonstrate the worst case ground vibration levels that are being achieved from current blasting operations. The centroid of the blasting operations is identified and radial lines constructed to the monitoring locations. Using a characteristic attenuation rate of 1.6, the milestone intercepts along the radial lines are determined. The 5, 2, 1 and 0.5 mm/s contours are then determined by connecting the intercepts. This worst case contour assessment is shown in **Appendices 1 & 2** - Ground Vibration Contour Assessment.

7.1.2. Predictive Model Approach

The worst case ground vibration can be analysed using the following Site Law model [1] by substituting for the measured values and determining K_v .

$$PPV = K_v \left(\frac{\sqrt{m}}{D} \right)^e$$

Where: PPV = Peak Particle Velocity (mm/s) [1]
 m = Charge mass per hole or per delay (kg)
 D = Distance from blast (m)
 k = Site constant
 e = The attenuation rate (1.6)

The ground vibration levels recorded during 2017 were used to calibrate the site constants used for predictive modelling, and these are listed in **Table 3**.

Table 3 – Site Constant Determinations

| Monitor | K_v |
|--------------|-------|
| Wright | 2420 |
| Watling | 2100 |
| Retreat | 1850 |
| Mines Rescue | 2150 |

Using the site constants (Kv) listed in Table 3, the distances at which “milestone” PPV levels that will occur in the directions between the North Pit or the West Pit blasting areas, and the Watling, Wright, Retreat, and Mines Rescue Station monitors can be calculated, and these are listed in Table 4.

The most significant “milestone” PPV levels are 5 mm/s and 10 mm/s.

Table 4 – Distances Related to Milestone PPV Levels

| PPV (mm/s) | Distance (m) | | | | | | | |
|---------------|----------------------------------|---------|---------|-----------------|----------------------------------|---------|---------|-----------------|
| | North Pit (charge mass = 500 kg) | | | | West Pit (charge mass = 1500 kg) | | | |
| | Wright | Retreat | Watling | Mines Rescue | Wright | Retreat | Watling | Mines Rescue |
| 10 | 691 | 584 | 632 | 660 | 1197 | 1010 | 1095 | 1143 |
| 5 | 1065 | 901 | 975 | 1017 | 1845 | 1560 | 1688 | 1763 |
| 2 | 1889 | 1597 | 1728 | 1805 | 3272 | 2770 | 2994 | 3126 |
| 1 | 2913 | 2463 | 2666 | 2783 | 5046 | 4270 | 4617 | 4821 |
| 0.5 | 4493 | 3799 | 4112 | 4293 | 7782 | 6580 | 7122 | 7435 |

The minimum distances between the either the North Pit or the West Pit blasting areas, and the Watling, Wright, Retreat, and Mines Rescue Station (M.R.S.) monitors, and the PPV levels that will result at those monitors using the worst case Kv values listed in Tables 5A and 5B: It may be seen that the PPV levels predicted at any sensitive site will be less than the 5 mms (95%) limit.

Table 5A – Highest PPV Levels that will result at Watling, Wright, Retreat, and M.R.S. Monitors North Pit

| Monitor | North Pit (Charge mass = 500 kg) | | | |
|-----------------------------------|----------------------------------|-------|--------|---------|
| | Retreat | M.R.S | Wright | Watling |
| Minimum Distance (m) | 2284 | 3110 | 3541 | 4895 |
| PPV Level at min. distance (mm/s) | 1.13 | 0.84 | 0.76 | 0.38 |

Table 5B – Highest PPV levels that will result at Watling, Wright, Retreat, and M.R.S. Monitors West Pit

| Monitor | West Pit (Charge mass = 1500 kg) | | | |
|-----------------------------------|----------------------------------|-------|--------|---------|
| | Retreat | M.R.S | Wright | Watling |
| Minimum Distance (m) | 3577 | 3883 | 2089 | 3662 |
| PPV Level at min. distance (mm/s) | 1.33 | 1.41 | 4.1 | 1.45 |

7.2 AIRBLAST OVERPRESSURE.

The peak airblast overpressure levels recorded during 2017 are listed in **Table 6**.

Table 6 – Peak Recorded Levels 2017

| Peak Airblast Level | | | |
|---------------------|---------|--------------|---------|
| Watling | Wright | Mines Rescue | Retreat |
| 110 dBL | 110 dBL | 110 dBL | 115 dBL |

The airblast overpressure levels recorded during 2017 were used to calibrate the site constants used for predictive modelling.

7.2.1. Predictive Airblast Model.

Terrock has developed the following predictive airblast models for prediction of airblast for various charge mass, burden, and stemming height specifications.

The airblast due to burden emission can be predicted from:

$$D_{115} = \left(\frac{ka \times d}{B} \right)^{2.5} \cdot \sqrt[3]{m}$$

Where: D_{115} = Distance to the 115 dBL contour
 d = hole diameter (mm)
 m = charge mass/hole (kg)
 B = face burden (mm)
 ka = a site constant

This model is used in conjunction with a regression line using 9 dBL with doubling of distance as the attenuation rate. The airblast contours resulting from a face blast are elliptical with the airblast directly in front of a blast using 6-10 dBL higher than for the same distance behind or at the side of a blast.

The airblast due to stemming column emission is predicted from:

$$D_{115} = \left(\frac{ka \times d}{S.H.} \right)^{2.5} \cdot \sqrt[3]{m}$$

Where: $S.H.$ = stemming height (m)

The airblast overpressure contours for these blasts are circular (equal emissions) in all directions.

Site calibration using the Rix's Creek data for 2017 gives the following models:

Burden emission model:
$$D_{115} = \left(\frac{170 \times d}{B} \right)^{2.5} \cdot \sqrt[3]{m}$$

Stemming emission model:
$$D_{115} = \left(\frac{130 \times d}{S.H.} \right)^{2.5} \cdot \sqrt[3]{m}$$

The stemming emission model predicted the highest levels, and these were used for this current assessment.

A centroidal contour approach was considered to be the most effective to analyse the results of airblast overpressure from current blasting operations.

The contours of the peak airblast are shown in **Appendix 2 – Airblast Contour Assessment**.

The milestone airblast levels for the peak airblast (PAV) are listed in **Table 7**

Table 7 – Milestone Airblast Overpressure Distances

| PAV (dBL) | Distance (front) (m) | Distance (rear) (m) |
|-----------|----------------------|---------------------|
| 115 | 1950 | 1250 |
| 110 | 2900 | 1820 |
| 105 | 4300 | 2800 |
| 100 | 6200 | 3900 |

The maximum airblast overpressure levels that will result at the minimum separation distances are listed in **Table 8**.

Table 8 – Peak Airblast Overpressure Predictions

| | Monitor | Minimum Separation Distance (m) | Peak levels predicted at closest dist.(dBL) |
|-----------|--------------|---------------------------------|---|
| West Pit | Wright | 2089 | 112 |
| | Retreat | 3577 | 104 |
| | Mines Rescue | 3883 | 101 |
| | Watling | 3662 | 101 |
| North Pit | Wright | 3451 | 100 |
| | Retreat | 2284 | 97 |
| | Mines Rescue | 3110 | 89 |
| | Watling | 4895 | 99 |

The airblast overpressure levels predicted at all sensitive sites will be less than the 115 dBL (5%) limit.

7.3 FLYROCK

Flyrock throw and trajectory path can be predicted by the use of the Terrock Flyrock Models:

7.3.1. Burden Control

$$L_{max} = \frac{Kf^2}{g} \left(\frac{\sqrt{M}}{B} \right)^{2.6} \quad \text{Where: } \begin{array}{ll} L_{max} = & \text{Maximum throw (m)} \\ g = & \text{Gravitational constant (g)} \\ M = & \text{Charge mass (kg/m)} \\ B = & \text{Face burden (mm)} \\ Kf = & \text{Flyrock constant} \end{array} \quad [2]$$

(Interim = 13.5 for coal overburden)

L_{max} occurs when the launch angle is 45°

7.3.3. Stemming Height Control

$$L_{max} = \frac{Kf^2}{g} \left(\frac{\sqrt{M}}{S.H.} \right)^{2.6} \sin 2\phi \quad \begin{array}{ll} S.H. = & \text{Stemming height (m)} \\ \phi = & \text{Launch Angle} \end{array} \quad [3]$$

= hole angle + 10° divergence

Kf could be calibrated for the Rix's Creek Site by a program of video review and flyrock throw measurement.

7.3.4. Burden Control Specifications (in front of face)

For Heavy ANFO 1.3g/cc density; M = 53 kg/m *B = 5m*

$$L_{max} = \frac{13.5^2}{9.8} \left(\frac{\sqrt{53}}{5} \right)^{2.6} = 50\text{m}$$

The minimum recommended exclusion zone in front of face becomes:

- Plant and Equipment: Safety Factor **2.0** Minimum Exclusion Zone = 100m
- Personnel, boundaries etc: Safety Factor **4.0** Minimum Exclusion Zone = 200m

7.3.5. Stemming Height Control (at sides and behind blast)

(i) Full Scaled Blasts

For Heavy ANFO 1.3g/cc density; M = 53 kg/m *B = 5m* *S.H. = 5000* *10° holes*

$$L_{max} = \frac{13.5^2}{9.8} \left(\frac{\sqrt{53}}{5} \right)^{2.6} \sin 140^\circ$$

= 32.1m (at high trajectory)

Minimum Exclusion: S.F. **2.0** = 65m

S.F. **4.0** = 130m

(ii) Shallow Blasts

For Heavy ANFO 1.3g/cc density; Charge= 0.2m long = .2 x 53 = 10.6 kg S.H. = 2.3m

$$L_{max} = \frac{13.5^2}{9.8} \left(\frac{\sqrt{10.6}}{2.3} \right)^{2.6} \sin 160^\circ$$
$$= 16\text{m}$$

Minimum Exclusion: S.F. **2.0** = 32m

S.F. **4.0** = 64m

The predicted flyrock trajectory paths are shown in **Figure 3a** and **3b**.

Current operating practice is to stop traffic on the New England Highway when blasting within 500m of the highway. This incorporates a substantial increase in the safety factors applied to the conservative Terrock flyrock model.

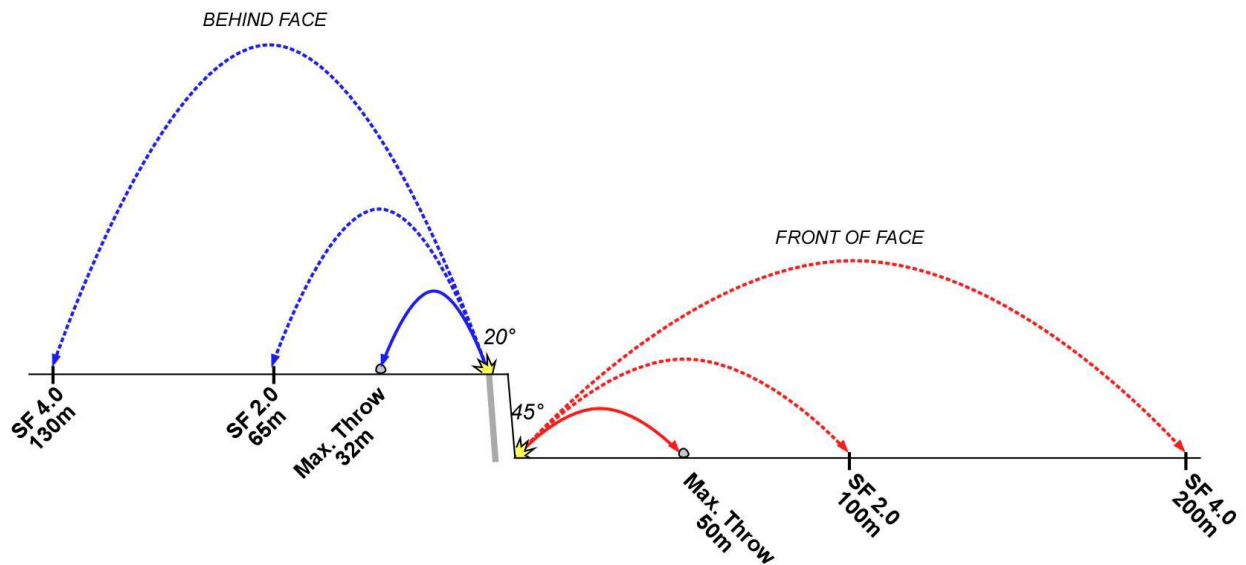


Figure 3a – Flyrock Trajectory Paths, 10° blast holes

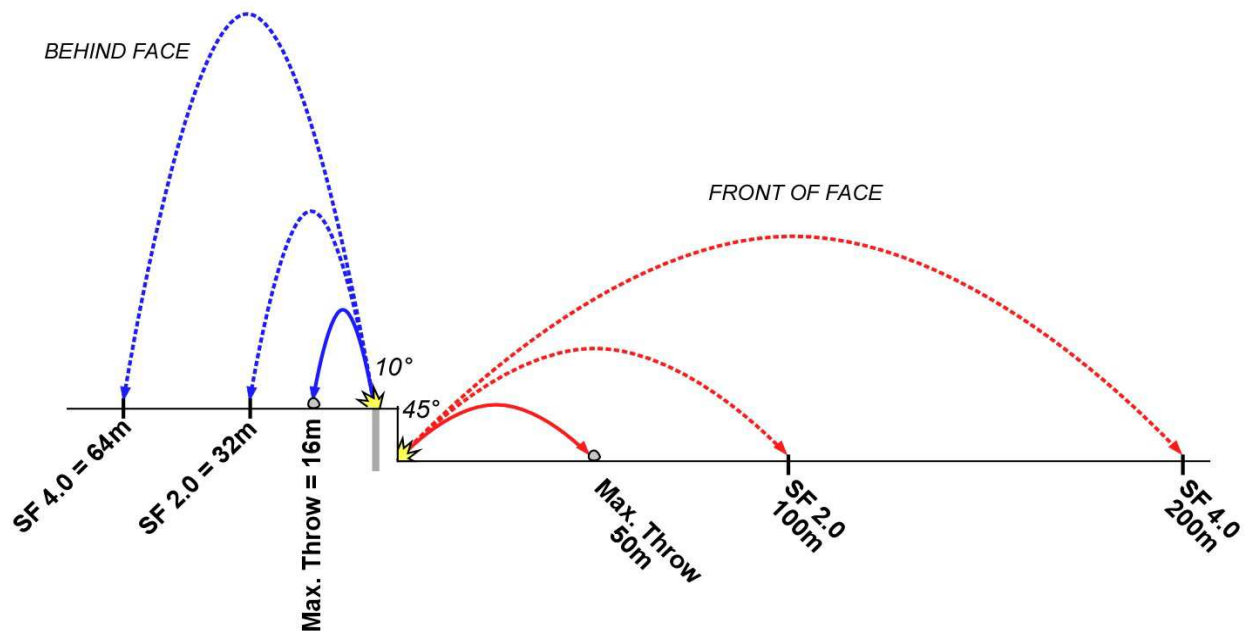


Figure 3b – Flyrock Trajectory Paths, vertical blast holes

8 EFFECT OF METEOROLOGICAL CONDITIONS ON ENVIRONMENTAL BLAST IMPACTS

8.1 METEOROLOGICAL DATA

Meteorological conditions can have a significant effect on airblast overpressure, dust and fume emission.

Control systems that have been developed for use in Hunter Valley open-pit coal mines constitute worlds-best-practice, and Rix's Creek Colliery has strongly supported these developments.

Predictive meteorological data that provides details of temperature and wind velocity at levels of up to 800 metres above the ground is produced by the Hunter Valley Meteorological Sounding Group (HVMSG), a joint venture between Hunter Valley coal mining companies of which Rix's Creek is a founding member. Examples of predictive HVMSG outputs are shown in **Figure 4**.

This meteorological data is used as inputs into models that are used to predict and assess the effect of meteorology on airblast overpressure, dust, and fume emission.

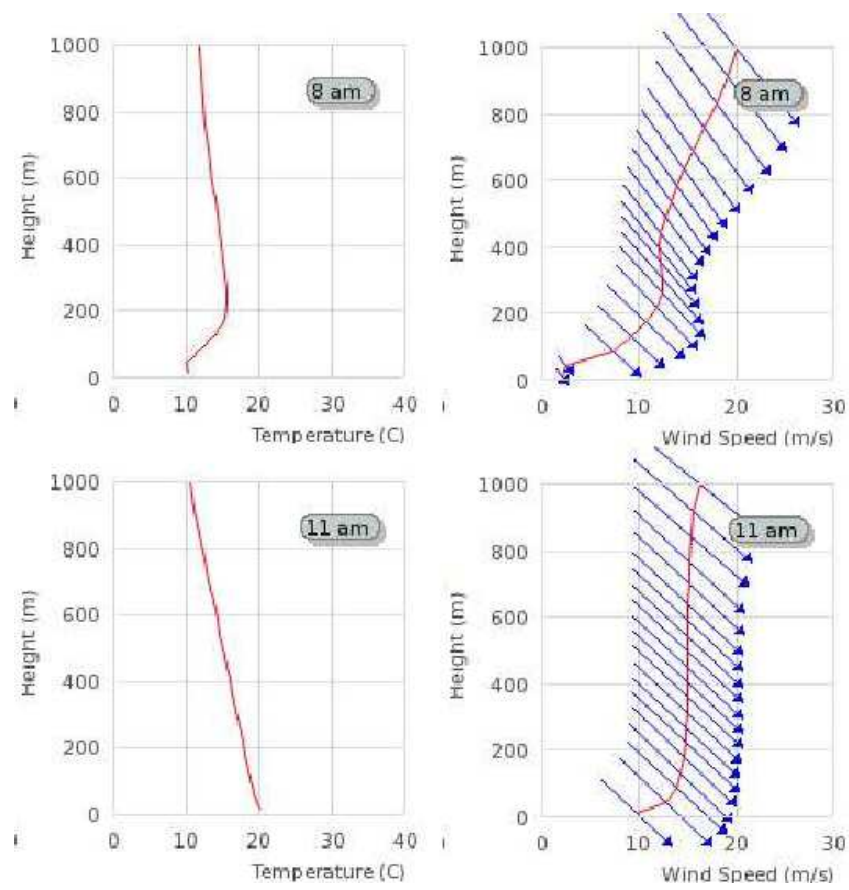


Figure 4 –Predictive HVMSG Data Outputs for 8am and 11am

8.2 AIRBLAST OVERPRESSURE

Rix's Creek (Rix's Creek and Rix's Creek North combined) uses the EnvMet airblast assessment system to predict the effects of meteorology on airblast overpressure. At 7am each morning, predictive outputs are available that give details of any increases in airblast overpressure levels that will result in the area surrounding the mine. An example of these outputs, which are provided at half-hourly intervals, is shown in **Figure 6** below.

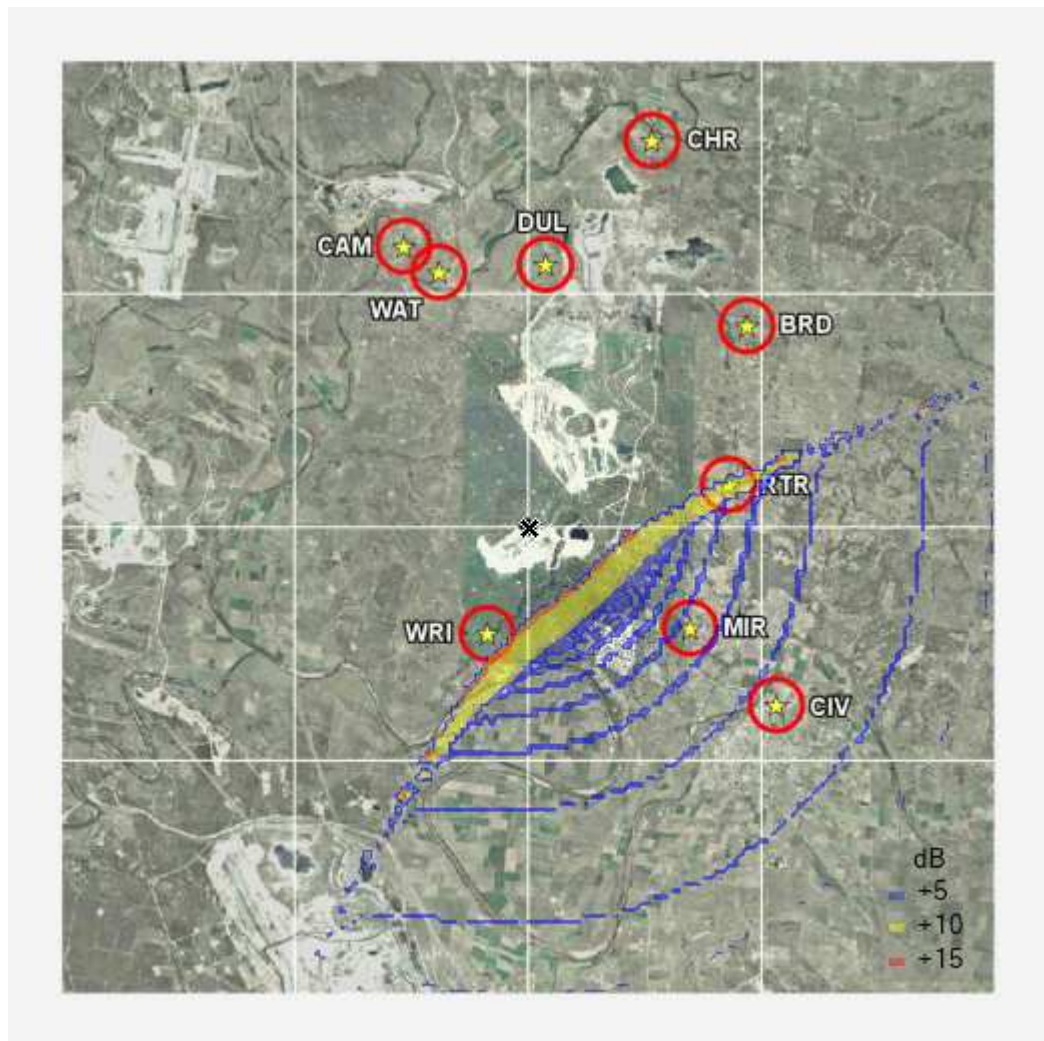


Figure 3 – Increase due to meteorology

The EnvMet system is also used to predict the basic emission levels that will result due to the blast design, as well as providing a prediction of the effect of basic blast emission and meteorological effects. Details of these outputs are shown in **Figures 7 and 8**:

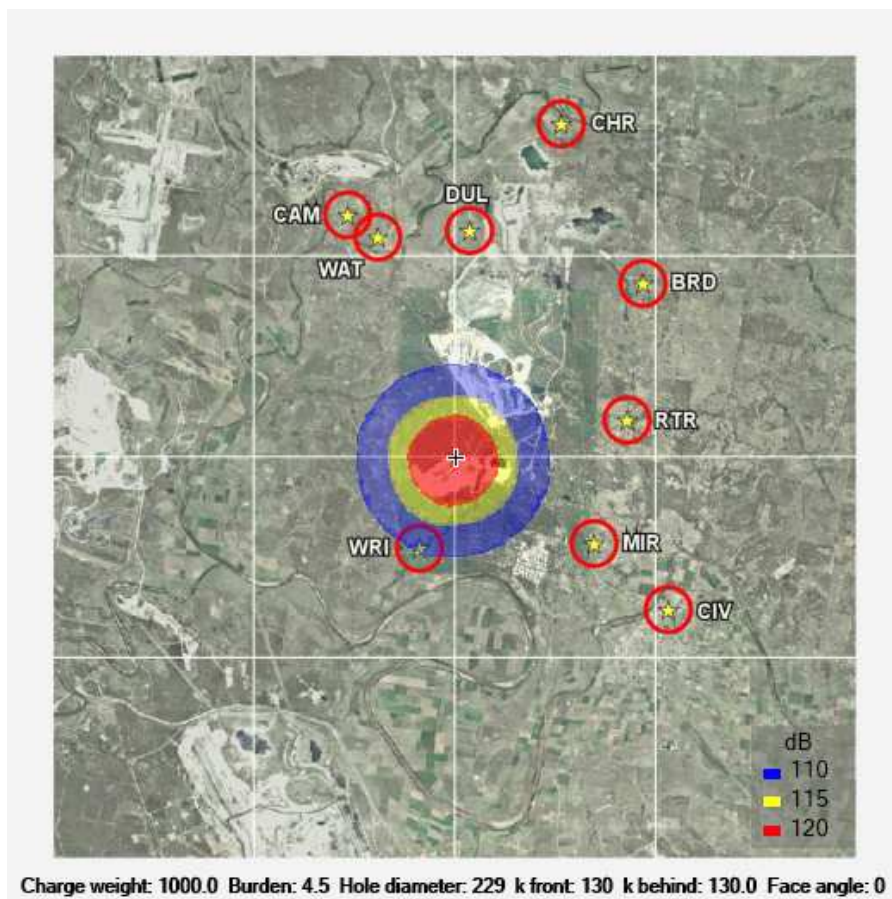


Figure 4 – Basic Emission

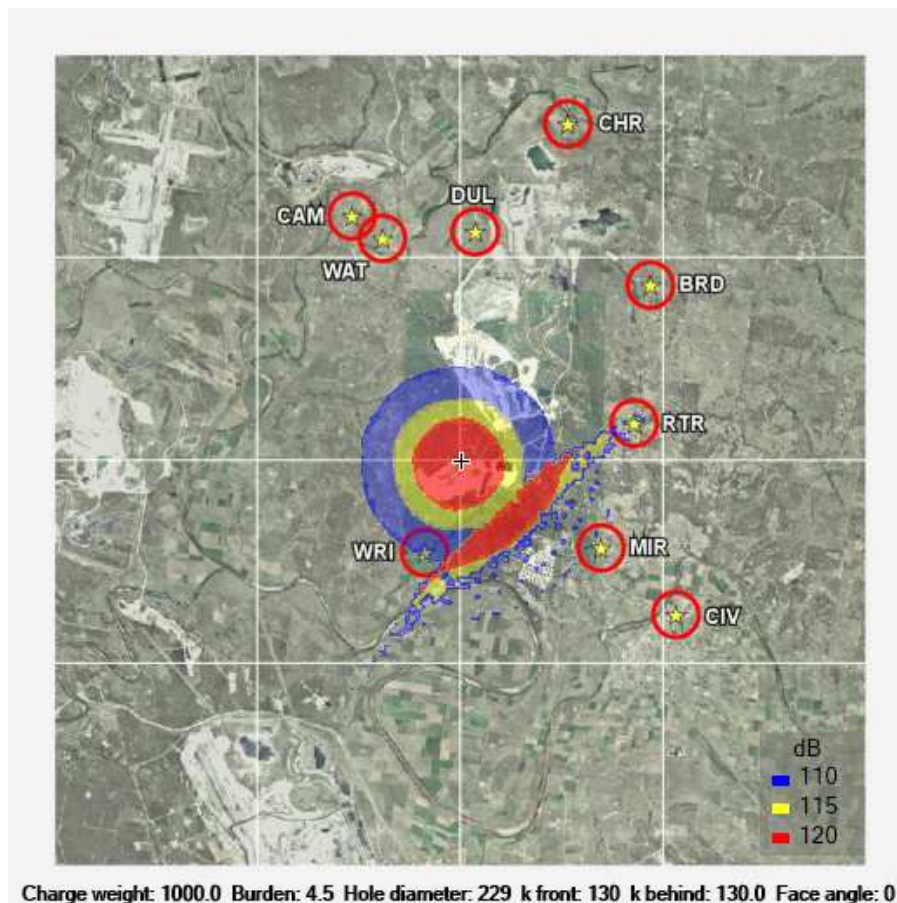


Figure 5 - Combined effect of basic emission and meteorological enhancement

8.3 DUST AND FUME EMISSION

Meteorological conditions strongly influence the control of dust and fume plumes.

Rix's Creek has responded rapidly to the need for an effective plume management system and uses a very effective plume modelling system that was developed by Todoroski Air Sciences. Inputs into the system is information about the size of the plume that is created by the blast, meteorological data provided by the HVMSG which is further refined by the Todoroski system, and detailed topographical information.

8.3.1. Fumes

There are two aspects involved in controlling the effect of fumes resulting from blasting.

These are:

- Limiting the amount of fumes that are emitted from the blast to form a fume plume
- Predicting the movement of the fume plume resulting from a blast, and ensuring that the movement of the plume does not result in fume concentrations that exceed permitted levels at sensitive locations.

8.3.2. Fume Emissions from a Blast

Factors influencing fume emissions resulting from a blast include:

- Explosives specifications
- Confinement
- Ground conditions
- The length of time that the explosives remain in the ground before firing.

It is not possible to control these factors precisely. Even minor variations in the characteristics of the chemicals used to make the explosives may result in an increase in fume emission. The degree of fume emission may also increase as ground conditions, including the type and amount of groundwater, vary.

At Rix's Creek Mine, methods such as minimising the 'sleep' time that explosive charges remain in the ground before firing have been developed.

Although precise prediction is not yet possible, a sufficient degree of correlation between significant factors and the amount of fumes produced has developed at Rix's Creek to permit the possibility of fumes resulting from each blast to be predicted using three categories (low, medium, and high).

8.3.3. Fume Plume Movement

The fume plume management modelling system used at Rix's Creek Mine quantifies the size of the fume plume produced from low/medium/ high emission blasts, and then predicts the movement of the plume.

8.3.4. Blast Dust Control

The system, which is used in conjunction with the fume plume system quantifies the size of the dust plume produced, and then predicts the movement of the plume. Although the amount of dust produced by different blasts will vary, the current system assumes worst case conditions.

8.4 USE OF THE FUME/DUST MODELLING SYSTEM

At 7am each morning predictive outputs are available that give details of the dust and fume plume emissions that will result in the area surrounding the mine. Details of these outputs, which are provided at hourly intervals, are shown in **Figure 9** below.

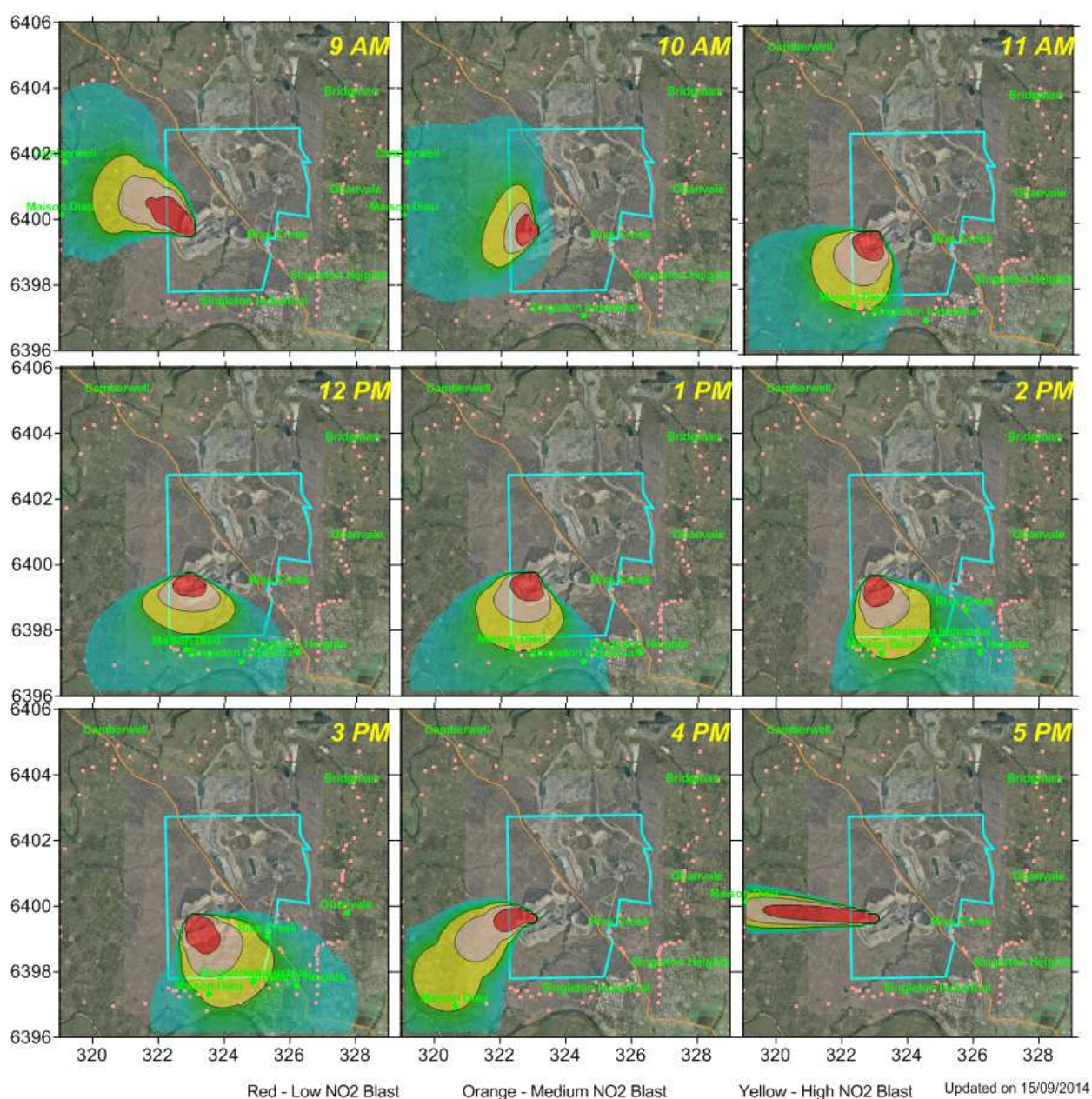


Figure 6 – Fume & Dust Plume Modelling Outputs

Low, medium, and high fume plume emissions are shown in red, pink, and yellow respectively, and a conservative assessment of the maximum dust plume is shown in blue.

Calibration of the dust and fume model was undertaken using App-Tek, model OdaLog Type 7000 gas analysers. Rix's Creek continues to use these gas detectors to monitor for blast gas fumes. This instrumentation allows for continuous refinement of the blasting practices on site.

9 OTHER BLASTING ISSUES

9.1 HIGHWAY STABILITY

The location of the New England Highway in relation to the Continuation Areas is shown in **Figure 10**.

There have been no problems with highway stability in the Singleton area when the underlying rock structure is gently sloping and stable.

Previous mining operations in the Rix's Creek mine north pit area adjacent to the New England Highway from the commencement of mining until 2010 was in stable, gently sloping rock structure. Mining was carried out to within 100 metres of the highway, and the resulting highwall was (and still is) stable.

Most mining in the continuation area adjacent to the New England Highway will be in the same stable rock structure, and there will be no modification to normal blasting practice required to ensure highway stability.

From 2010 – 2014, mining operations in the area adjacent to the New England Highway at the northern end of the north pit were in ground with a steeply sloping rock structure, and modifications to mining practice, including advanced placement of backfill, were required to ensure highway stability.

There is a limited area at the northern end of the western pit in the continuation area where the rock structure will require modifications to mining practice. Further details regarding this are given in the Geotechnical Report.

9.2 HISTORIC COKING OVENS

The location of the Coking Ovens are shown in **Figure 10**.

The ground vibration PPV limits at the historical Coking Ovens are:

- ≤ 5 mm/s for 95% of blasts
- ≤ 10 mm/s for all blasts.

Controlling ground vibration to these limits from North Pit blasts in the vicinity of the Coking Ovens has been achieved in the past by environmental blast design. The methodology is:

- Establish the predictive Site Law

$$PPV = K_v \left(\frac{\sqrt{m}}{D} \right)^e$$

Where: PPV = Peak Particle Velocity (mm/s) [1]
 m = Charge mass per hole or per delay (kg)
 D = Distance from blast (m)
 k = Site constant
 e = The attenuation rate (1.6)

- Use the Site Law to determine the charge mass limit appropriate for the distance from the blast to the Coking Ovens;
- To achieve the ground vibration target, it may be necessary to limit the charge mass by such means as:
 - Using a less dense explosive
 - Decking the explosive column
 - Using air decks.

The Site Law at the Coking Ovens should be reviewed by analysis of the measurements and modified if necessary for future blast design.

9.3 BLASTING IMPACT ON EXTERNALLY OWNED/MANAGED INFRASTRUCTURE

Externally owned/managed infrastructure that could be considered as being possibly impacted by blasting in the continuation area are shown in **Figure 10** and includes:

- New England Highway
 - pavement and culverts
 - bridge over Rix's Creek
 - haul road bridge over the highway
 - cut and cover tunnel under the highway
 - A second proposed cut and cover tunnel
- Buried Fibre Optic Cable beside the highway

- Main Northern Rail Line
- 66Kv Ausgrid power line from Maison Dieu Industrial area traversing Rix's Creek Lane (concrete and timber poles) to the Rix's Creek Mine infrastructure
- A Dam certified by the Dam Safety Committed in the Rix's Creek – Northern Operations mine area.
- Other uncertified dams on the Rix's Creek – Southern Operations and Rix's Creek – Northern Operations mine areas.

The infrastructure associated with the New England Highway has been assessed and approved by RMS as part of the approval process.

The owner/managers of the fibre optic cable are aware of the project having been involved with design and installation of the cut and cover tunnels.

The Main Northern Railway Line is located over 1.3km from the nearest blasting in the North Pit of the Continuation Area. This affords sufficient separation that observance of the appropriate procedures and protocols of the Rail Track Authority for blasting closer than 600m is not necessary.

Ausgrid power lines - Ausgrid routinely applies a limit of 100mm/s on their poles. There is sufficient separation that compliance with their limit can be readily achieved by environmental blast design.

The Certified Possum Skin Dam in the Rix's Creek – Northern Operations property is over 4.0 km from the continuation area blasting so controlling ground vibration to the Dam Safety Committee limit is not an issue.

There are no specified vibration limits on the other dams not under the Dam Safety Committee regulation. The inspection regimes will continue to ascertain any change of condition.

9.4 BLASTING IMPACT ON OTHER MINES

The nearest mine to the continuation area is the Rix's Creek – Northern Operations mine which is owned and operated by Bloomfield.

The main infrastructure of the Rix's Creek – Northern Operations mine is over 3km from the nearest continuation area blasting and the predicted peak ground vibration levels are less than 0.3mm/s, which is at human threshold perception levels with no potential structural issues.

Blasting in the continuation area is a progression of blasting that has been conducted over many years and will impose no additional impacts on the Rix's Creek – Northern Operations mine compared to what has previously happened.

9.5 BLASTING IMPACTS ON LIVESTOCK

Blasting in the continuation areas is not expected to have any impact on live stock because, even in a new mine, the general experience is that domestic animals rapidly become acclimatised to blasting.

At the opening of the Bengalla open-cut coal mine near Muswellbrook, the behaviour of thoroughbred horses to the initial blasts was observed specifically because of concern of the stud owners. The horses were observed to look up momentarily after the blasts and then continued grazing. At the same mine, two commercial dairies (Wantana and Lumeah Dairies) operating on river flats of mine owned land reported that blasting had no adverse impacts on its cows which were exposed to blast vibration levels up to at least 10mm/s without detriment to milk production or animal welfare.

The continuation area is not a new mine and blasting is a progression of blasting that has occurred in the general area from the two adjoining mines over many years. The only possible impact of blasting would be on livestock brought into the area from a non mining area. The experience is that they will rapidly become accustomed once they perceive they are not threatened.

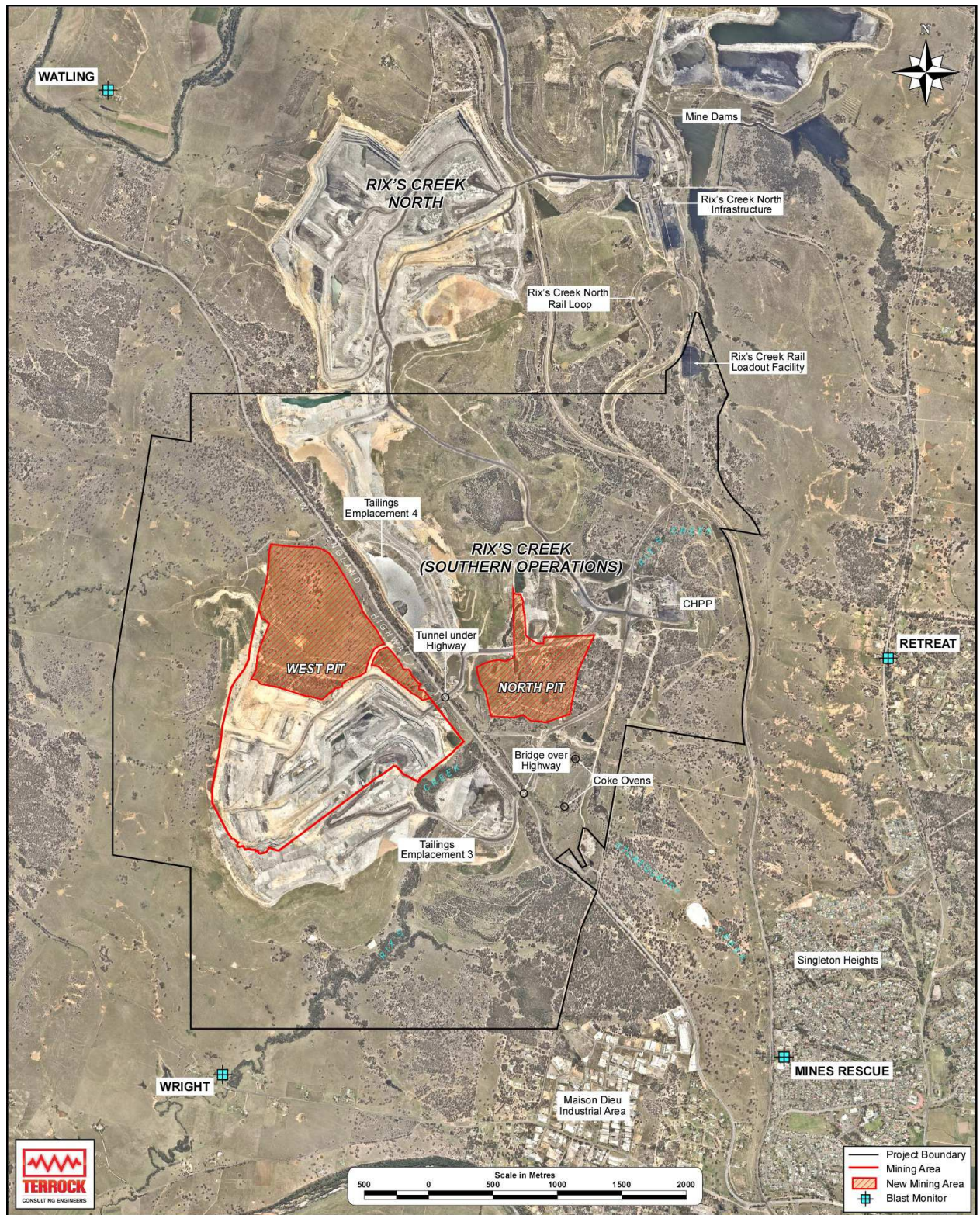


Figure 10 – Location Plan With Nearby Infrastructure

10 CONCLUDING COMMENTS

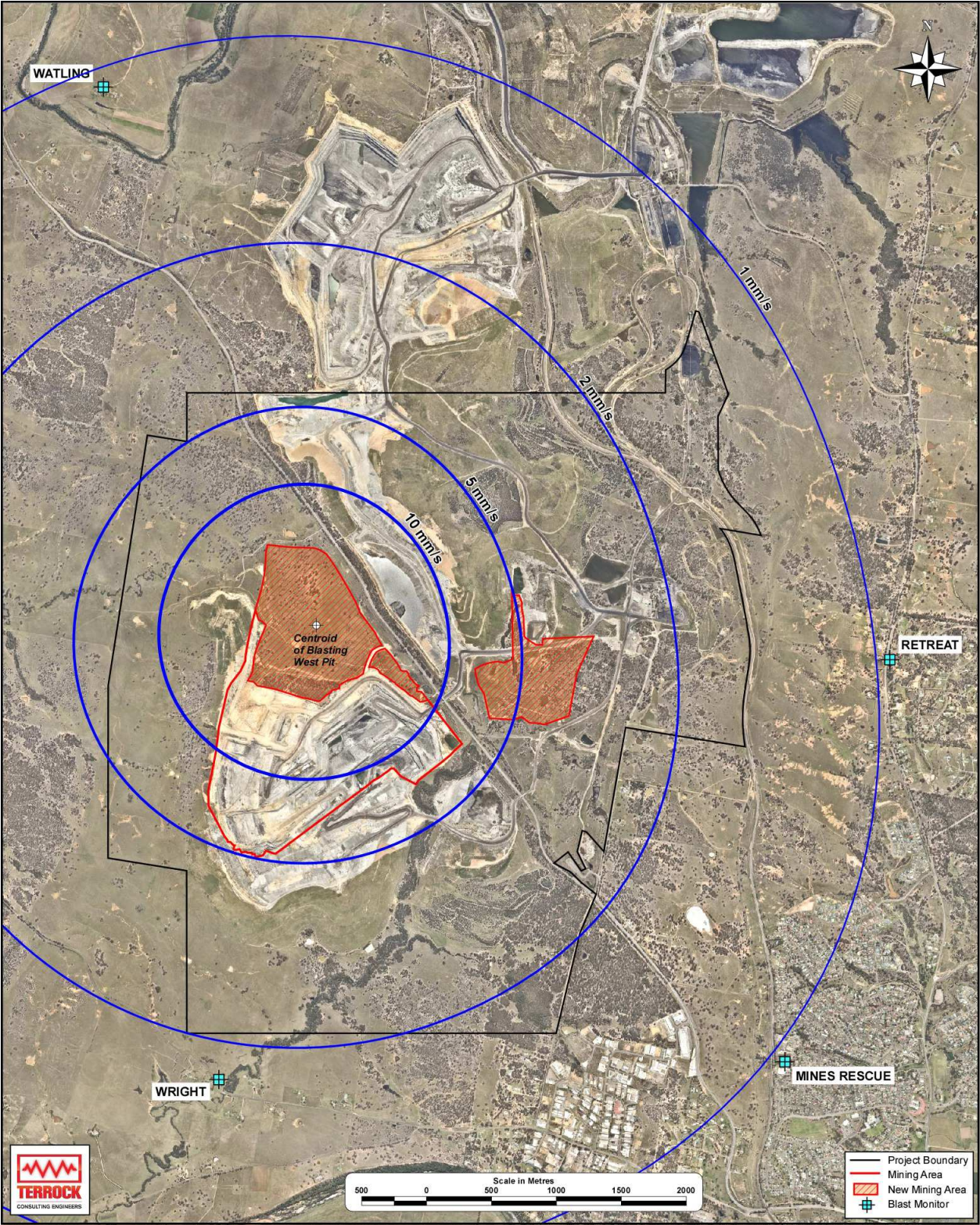
- The airblast overpressure and ground vibration levels from current blasting operations comply with regulatory limits at all sensitive sites.
- Both ground vibration and airblast overpressure levels from future blasts in the West Pit and North Pit of the continuation area will be below regulatory limits.
- Compliance with ground vibration limits at the Coking Ovens may further reduce the scale of blasting operations which will be reflected at Retreat and Mines Rescue by lower ground vibration and airblast overpressure than predicted.
- Dust and fumes are limited by the practices described in this report and further detailed in the Blast Management Plan.
- Mining in the area adjacent to the New England Highway will be predominantly in stable ground. Previous experience when blasting in stable ground in the northern pit has shown that no modification to normal blasting practice to ensure highway stability. There is a limited area at the northern end of the western pit in the continuation area where the rock structure will require modifications to blasting practice, and these will be applied as required.
- Flyrock can be readily controlled by appropriate blast design and loading practice, and if the recommended exclusion zones are observed, will not present a danger to personnel within the mine lease or outside the extraction area.
- Traffic control on the New England Highway will resume when blasting approaches closer than 500m to the Highway.
- Blasting in the continuation area will have no significant impact on nearby infrastructure.
- Blasting will continue to have no impact on livestock.



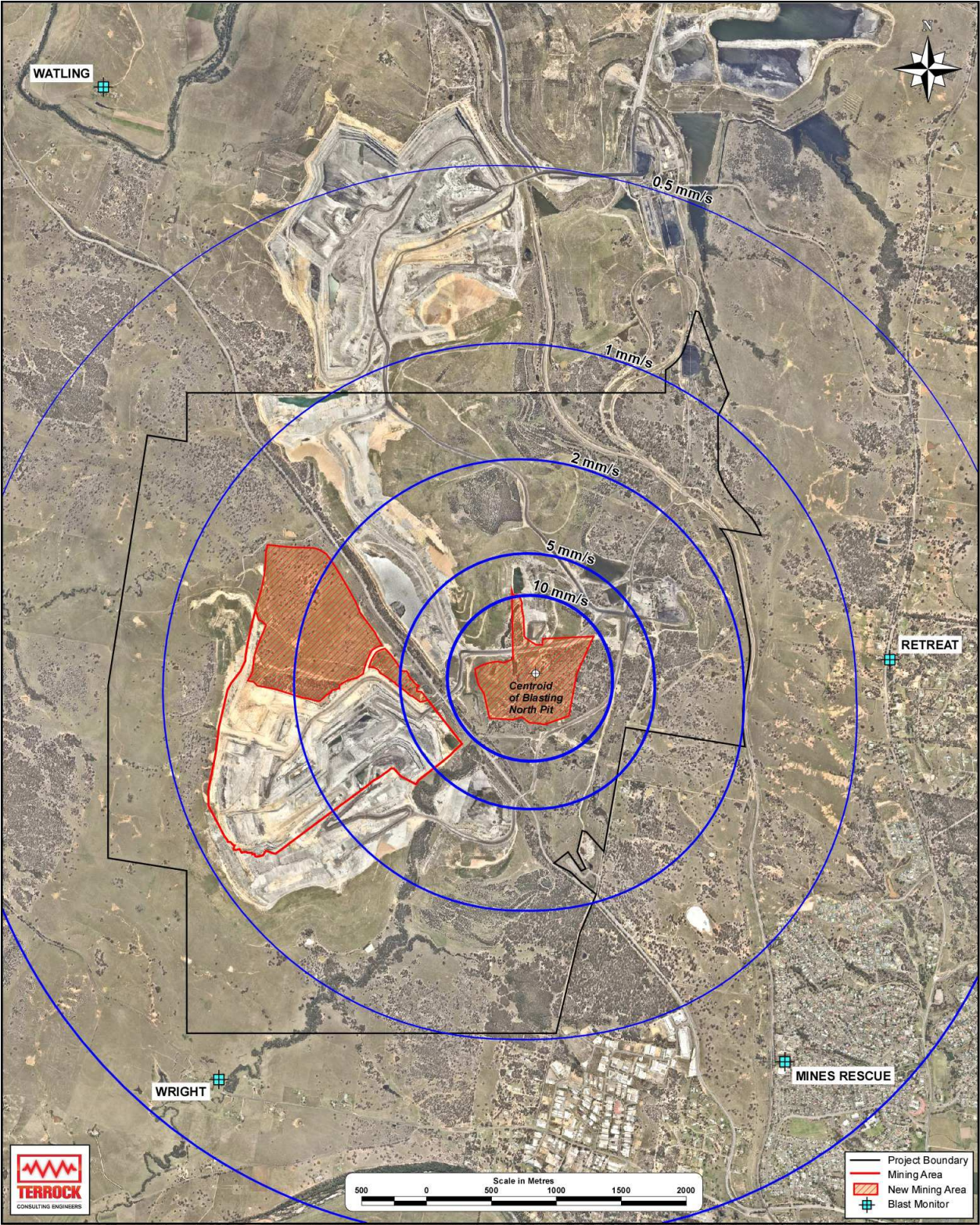
Alan B Richards
28th February 2018.

APPENDICES

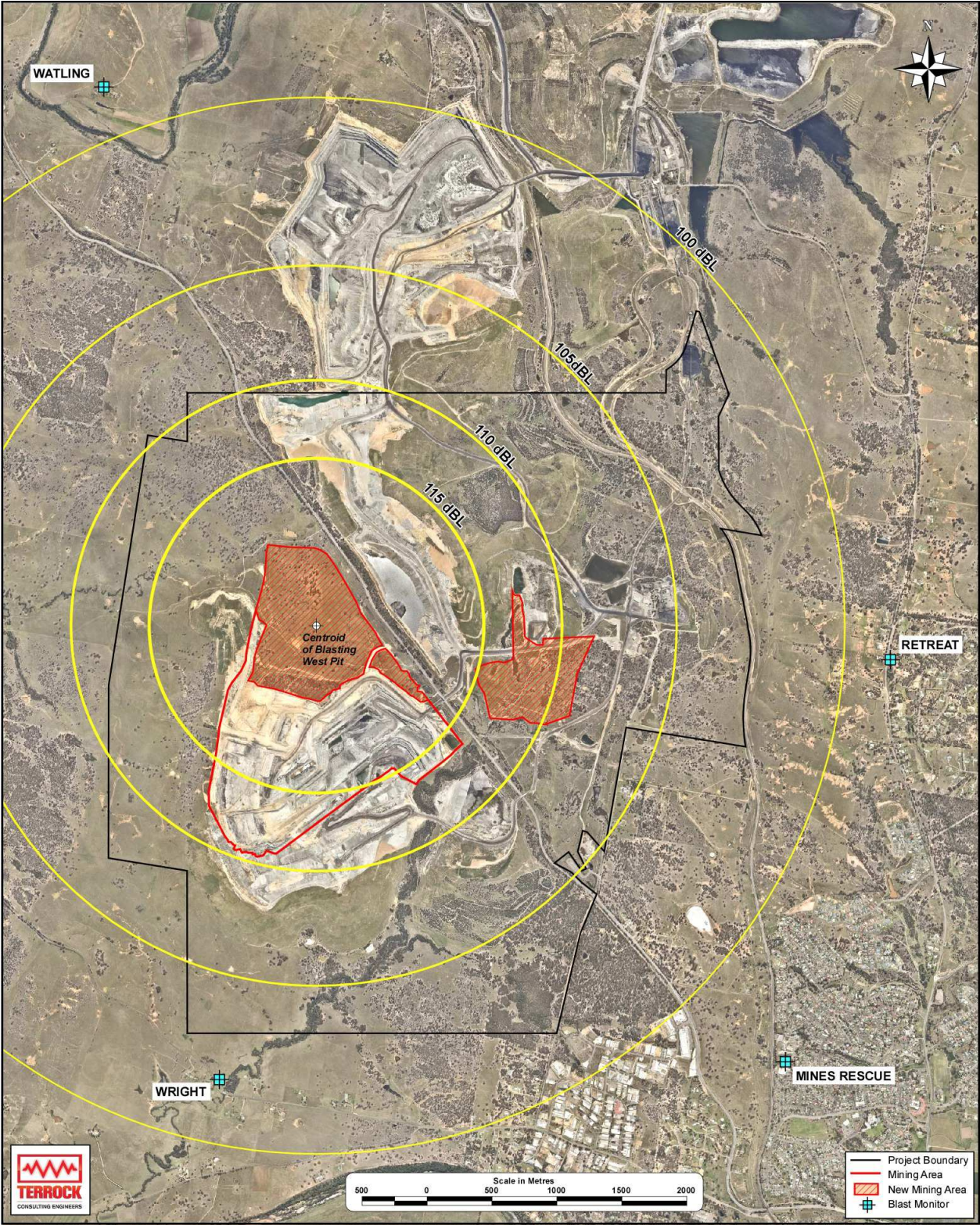
APPENDIX 1A - GROUND VIBRATION CONTOUR ASSESSMENT – WEST PIT



APPENDIX 1B - GROUND VIBRATION CONTOUR ASSESSMENT – NORTH PIT



APPENDIX 2A - AIRBLAST CONTOUR ASSESSMENT – WEST PIT



APPENDIX 2B - AIRBLAST CONTOUR ASSESSMENT – NORTH PIT

